Lecture 2 : PROBLEMATIC SOILS PROBLEMATIC SOILS IN INDONESIA AND LESSONS LEARNED FROM GEOTECHNICAL FAILURES



PRE CONFERENCE WORKSHOP SOIL IMPROVEMENT FOR MEGA INFRASTRUCTURE Paulus P. Rahardjo Professor of Geotechnical Engineering Universitas Katolik Parahyangan Bandung – INDONESIA



OUTLINE OF PRESENTATION

□ GENERAL UNDERSTANDING ON SOFT SOILS AND PROBLEMS IN CONSTRUCTION

- ORIGINS OF SOFT SOILS
- **CASE HISTORIES**
- LESSONS LEARNED AND SUMMARY

GENERAL UNDERSTANDING ON SOFT SOILS

Recent Sediments and Organics or Peats

Soils that become soft when exposed to water

Soils originated from Volcanic Ashes

Dispersive soils

Clayshales and Expansive Soils upon wetting Uncompacted fill soften due to water infiltration

ENGINEERING PROBLEMS OF CONSTRUCTION ON SOFT SOILS



Problems with low bearing capacity Problems with stability in excavation Problems with stability of embankment Problems with long term settlement Problems with constructability Problems with Negative Skin Friction Problems with Adjacent Structures □ etc

RELATED-ENGINEERING PROBLEMS OF CONSTRUCTION ON SOFT SOILS



- Problems with lack of understanding
- Problems with lack of experience
- Problems with lack of (geotechnical) data (reluctant to conduct soil investigation)
- Problems with ignorance
- Problems with cost limitation
- Problems with in-sufficient time of investigation
- Etc etc



ORIGINS OF SOFT SOILS

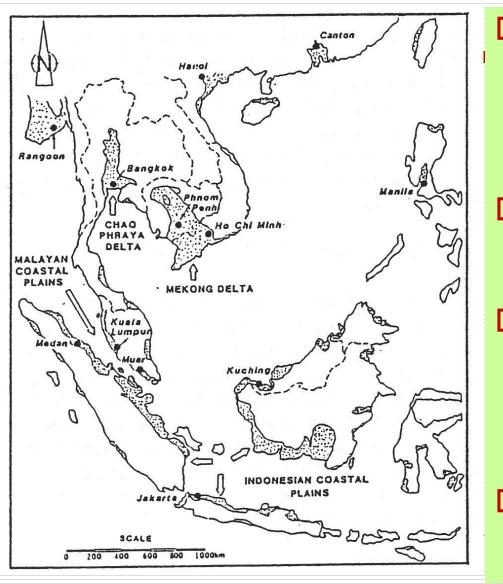
RECENT SEDIMENTS

LAKE DEPOSITS

□ MUD ERUPTIONS

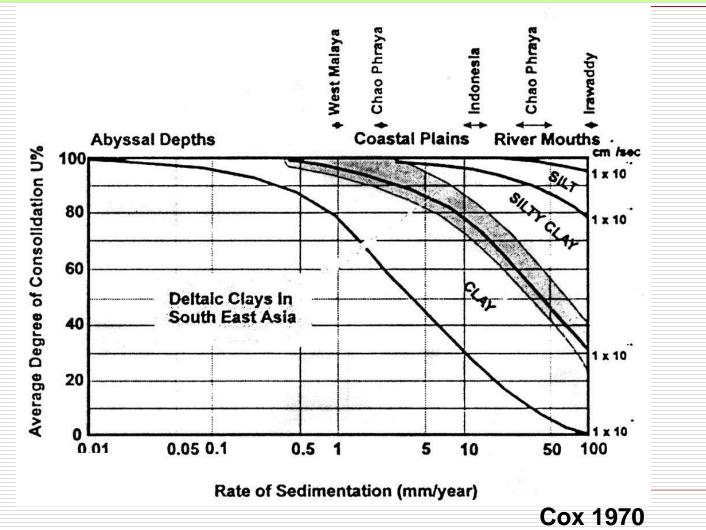
RECENT SEDIMENTS





- Soft soils are distributed mainly in coastal areas where big cities are being developed
- Construction on soft soils has risks of failures
- Most of failures are contributed by those who are not aware of the risk or lack of knowledge on soft soils
- Technology has been well developed to work out soft soils

Soft soils in South East Asia: young and consolidating

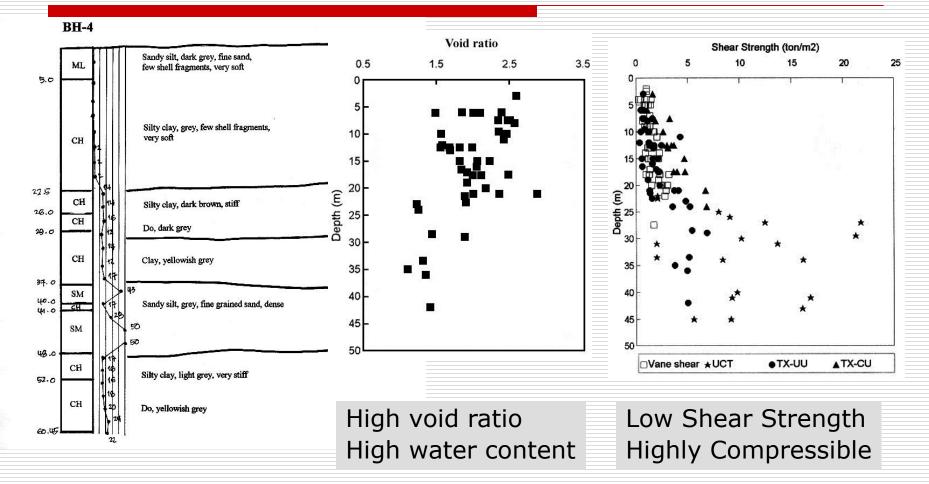


TRANS ATTON

Cable to Computer Electric Cone Perietrometer. 1. Saturation of Cone Tip Cavities with 607 Apex: and Placement of Pre-Saturated $d = 36 \text{ mm} (10 \text{ cm}^2)$ Porous Filter Element. οr Obtain Baseline Readings for $d = 44 \text{ mm} (15 \text{ cm}^2)$ Tip, Sleeve, Porewater Transducer, & Inclinometer Channels Continuous Hydraulic Push at 20 mm/s; Add Cone Penetration Test (CPT) rod every 1 m. per ASTMD 5778 procedures Cone Rod (36-mm diam.) Inclinometer f. = sleeve friction **Readings taken** u_n = porewater pressure every 10 to 50 mm đ, $a_n = net area ratio (from triaxial calibration)$ U_{k_1} q_c = measured tip stress or cone resistance 91 $q_i = corrected tip stress = q_i + (1-a_i)u_{in}$



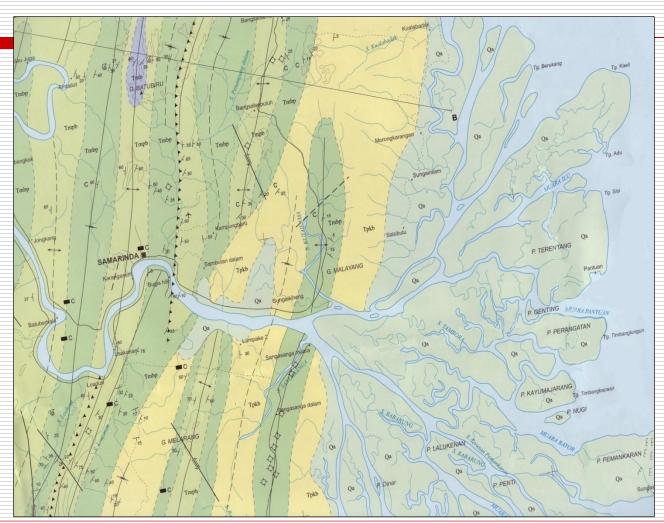
Example of Soft Soils in Indonesia North Semarang City (Central Java)



General Soft Soil Condition in Semarang

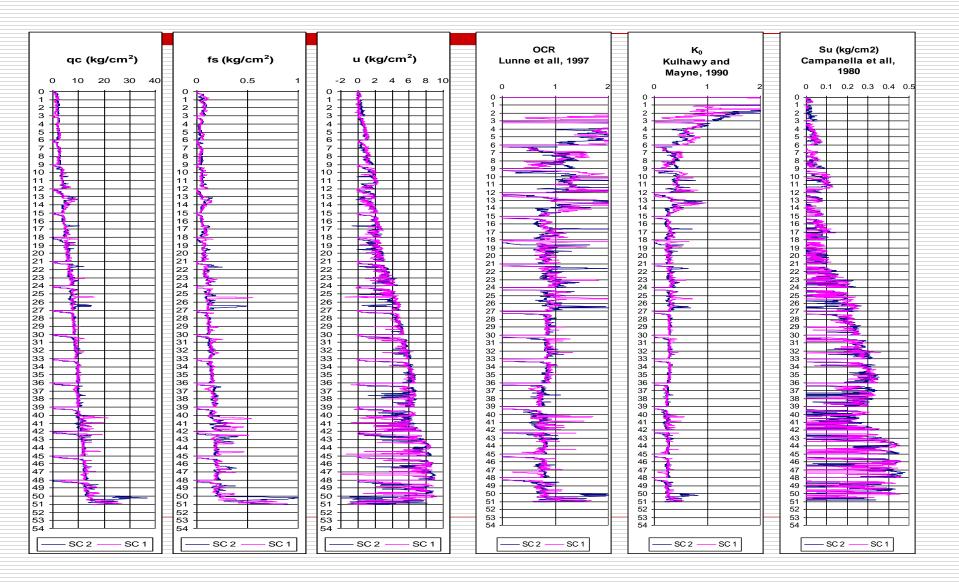
Examples of soft soils in INDONESIA Mahakam Delta – East Kalimantan





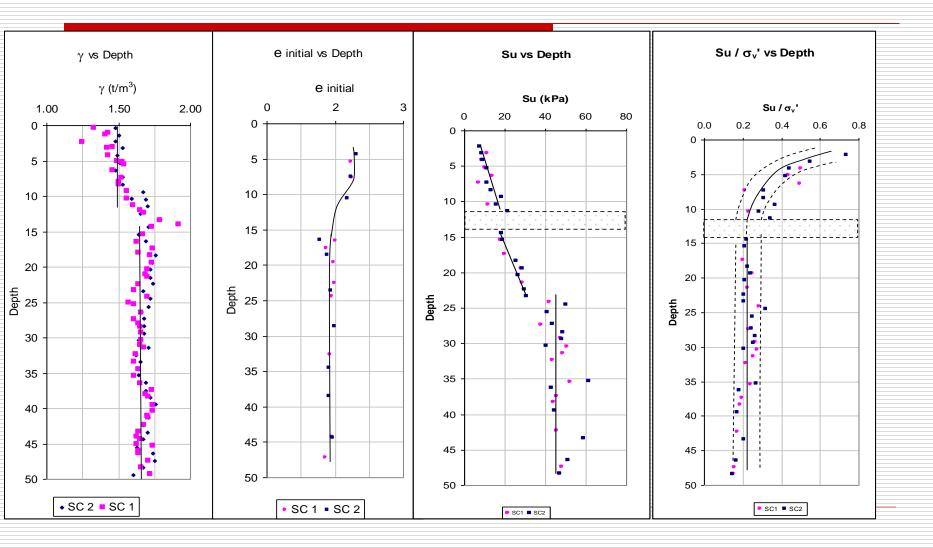
Mahakam River is the longest in Indonesia, Recent sediment of 50 – 80 m depth

Site Characterization of deep soft Mahakam deltaic soils using CPTu



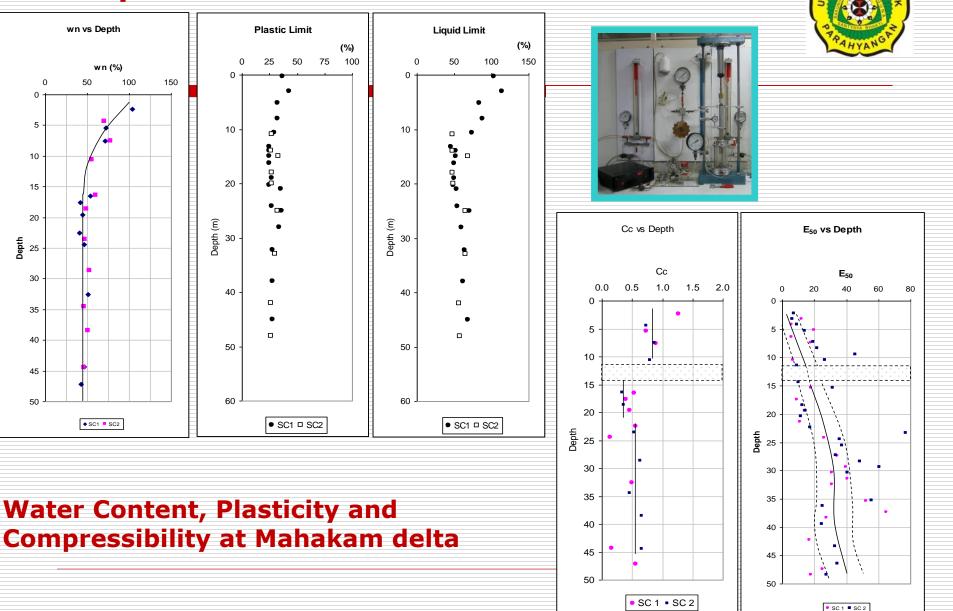
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Examples of soft soils in INDONESIA Mahakam Delta – East Kalimantan

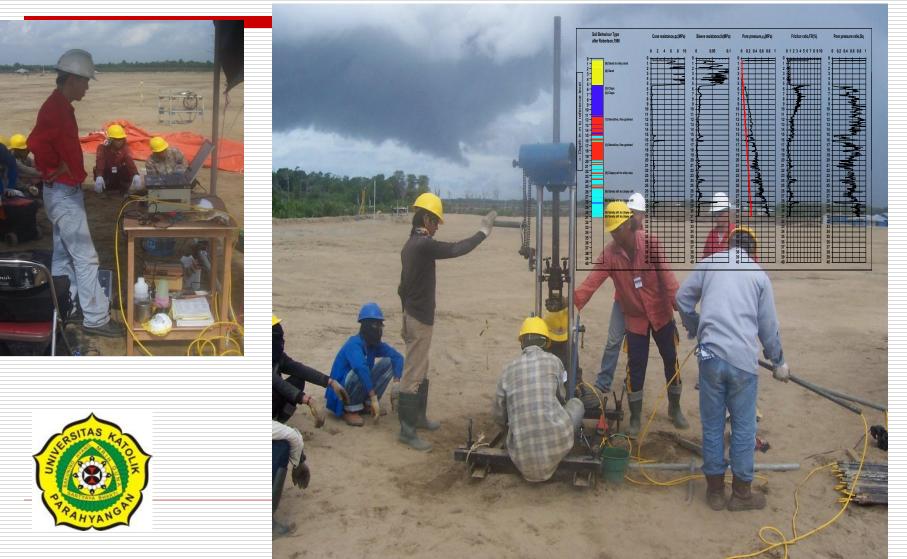


Void Ratios and Shear Strength

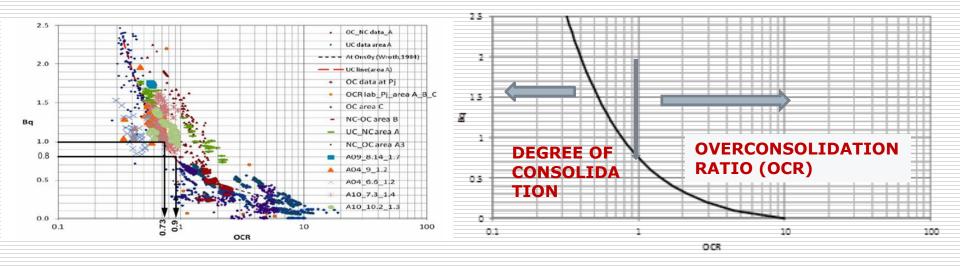
Example of Soft Soils in INDONESIA



Determination of the degree of consolidation of Reclaimed Site on deep soft Mahakam deltaic soils using CPTu



Method of Determination of the Degree of Consolidation Based on Bq from CPTu (Setionegoro, 2013 and Rahardjo et al 2014)

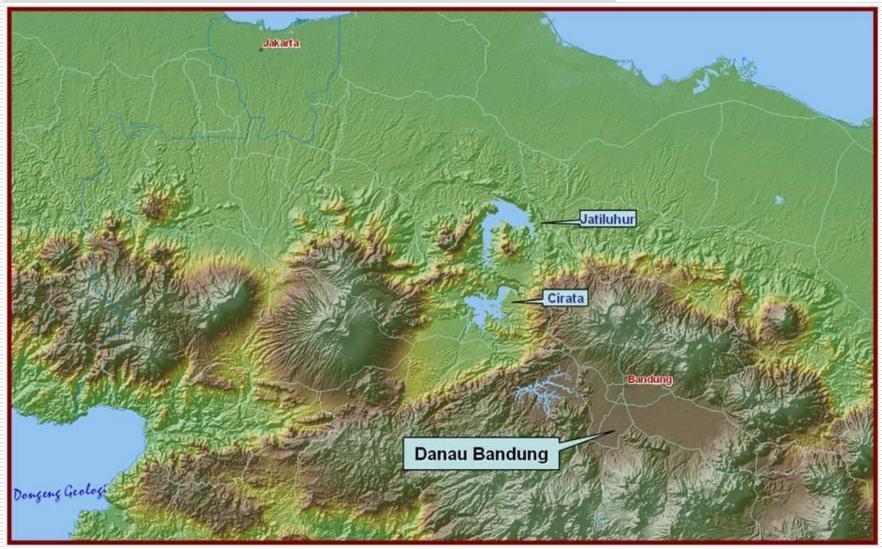


Setionegoro 2013

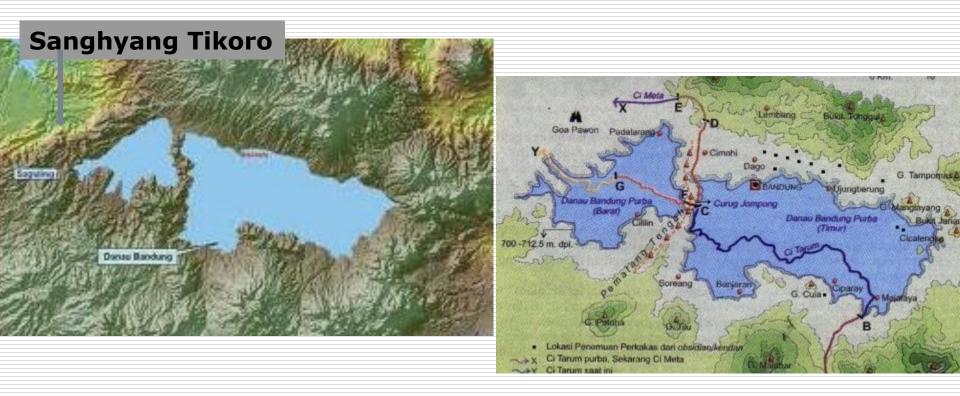
Rahardjo et al 2014



Bandung Lake Deposit



South Bandung was originated from ancient lake





Origin of Bandung Soft Soils were two ancient Lakes



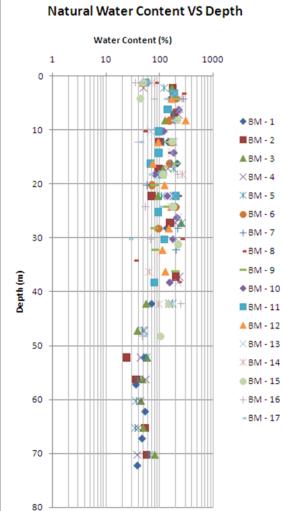




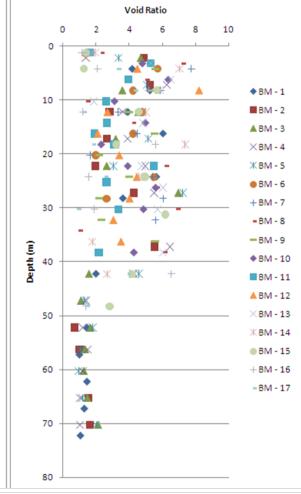
Bandung Soft Clays



Upper Layer (0-42m) Void Ratio 1.0-8.3 Water content 50-300%

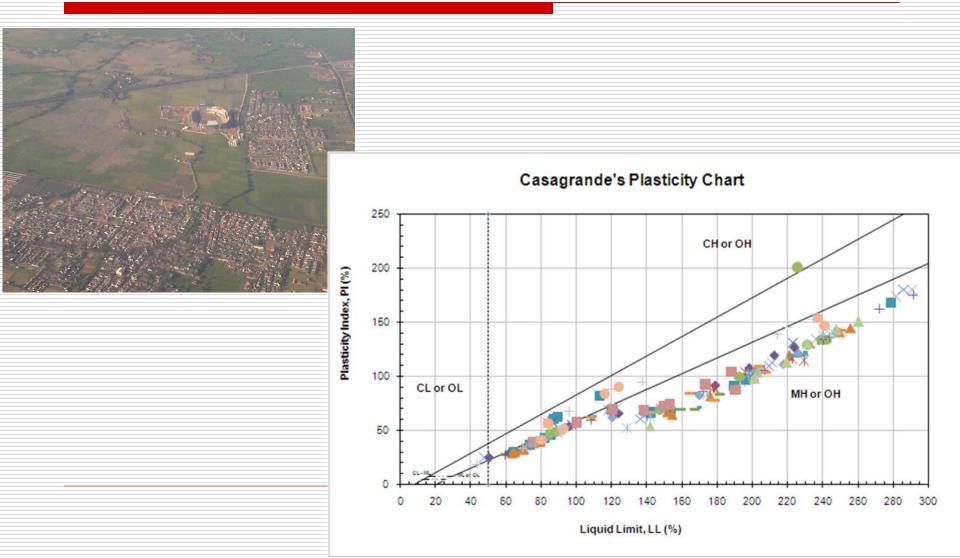


Void Ratio VS Depth



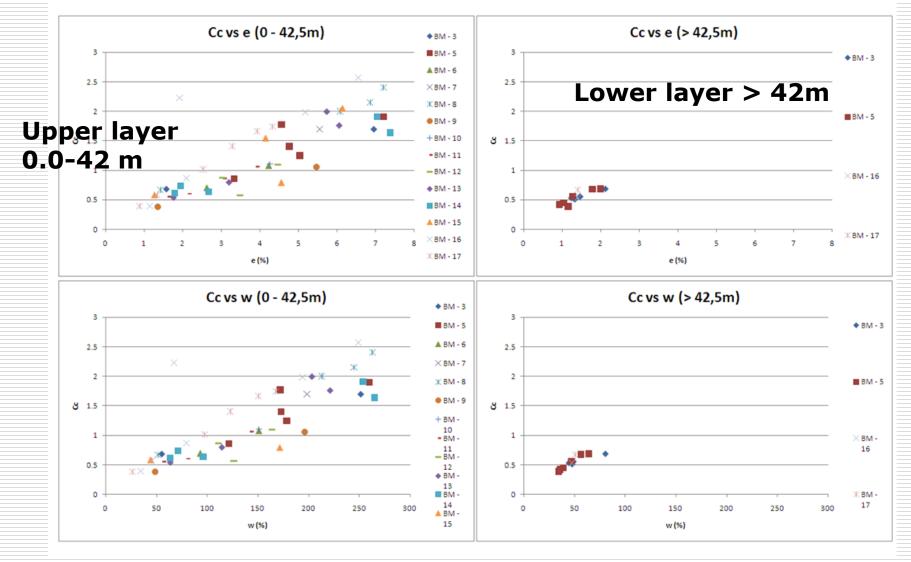


Bandung Soft Clays





Bandung Soft Clays



Other origin of soft soils Mud Eruption in East Java (Sidoardjo Mud – since May 29, 2006)



Site Condition of the Mud



Mud Eruption in East Java (Sidoardjo Mud)



Sidoardjo Mud created geotechnical problems



GEOTECHNICAL PROBLEMS

-Mud Removal

-Dyke Failures

-Settlement in Large areas

-Mud properties Difficult to handle



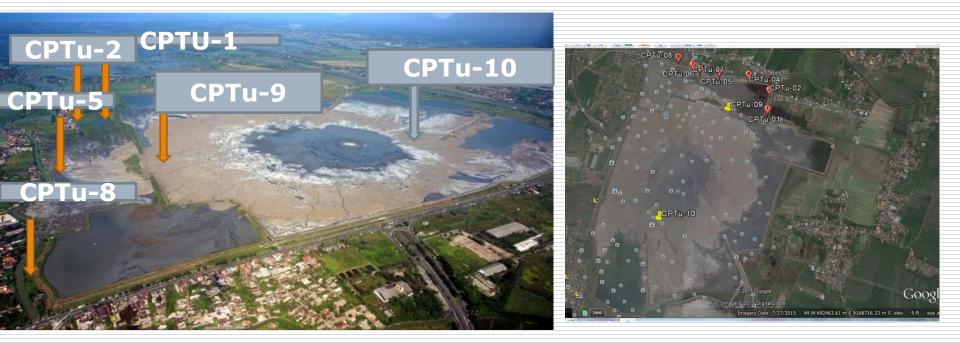






Investigation of Mud Eruption Disaster Area





Results of CPTu at MUD ERUPTION AREA

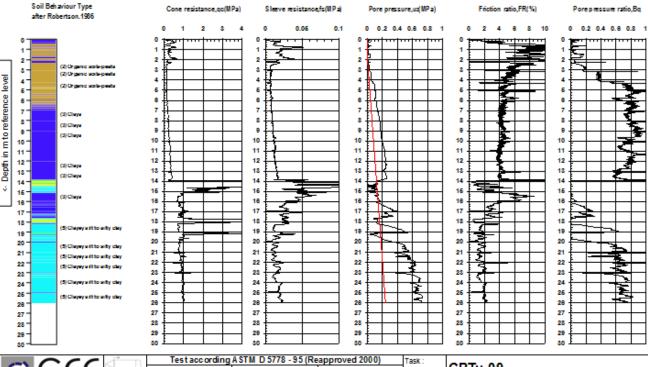




CPTu-9 : through the mud

Ground water -2.0 m 0-2 m : mud crust 6-14 m : mud 14-15m: silty sand (lenses) > 15m : soft clays

Results of CPTu at MUD ERUPTION AREA



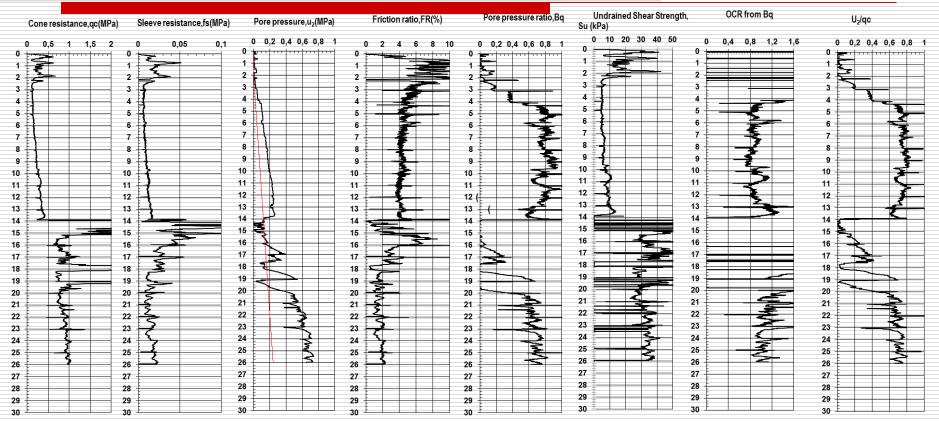
CPTu-9: through the mud Ground water -2.0 m 0-2 m : mud crust 6-14 m : mud 14-15m: silty sand (lenses) > 15m : soft clays

	Test accordin	Test according ASTM D 5778 - 95 (Reapproved 2000)							1	
C) ((🗀	g G.L:	W. L :	2 m	Pre Drill :	0 m		CPTu-09			
Project	d: CPTu Lumpur Lapin	do				Cone no. :	C10CFIIP.C12347	Date :	October 25, 2015	
Locati	tion : Sidoarjo					CPTno.:	CPTu-09	Operator :	AS/AW	
Phone: +02-22-3004072, Fax: +02-22-3006278 E-mail: 000:gents: Mail: POSID	ion:					Project no. :				





Interpretation of CPTu-09 (inside the dyke)





Mud Characterization by CPTu-10

CPTu-10



Results of CPTu at MUD ERUPTION AREA



Soil Behaviour Type after Robertson, 1986	Cone resistance.g d(MPa)	Sleeve resistance,fs(MPa)	Pore pressure µ a(MPa)	Friction ratio,FR(%)	Pore pressure ratio,Bq
1 2 2 2	1 1 2 1 3 1 4 1 6 1 11 1 12 1 13 1 14 1 15 1 16 1 17 1 18 1 19 2 20 2 21 2 22 2 23 2 24 2 25 2 26 2 27 2 28 2 29 3	0 0.06 0.1	0 0.2 0.4 0.8 0.8 1	0 2 4 8 3 10 0 1 2 4 8 3 10 1 4 6 8 7 8 8 9 10 10 1 12 1 13 1 14 1 14 1 14 1 15 1 16 1 17 1 18 1 19 1 10 1 10 1 10 1 11 1 12 1 13 1 14 1 14 1 15 1 16 1 17 1 18 1 19 1 10 10 1 10	0 0.2 0.4 0.8 0.3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
$O \leftrightarrow C$	GL: W.L:	0 m Pre Drill :	0 m	CPTu-10	

CPTu-10: through the mud Ground water -0.0 m 0-2 m : mud crust 2-21 m : mud (consolidating mud)

Bq = 0.8-1.2)

> 21m : soft clays

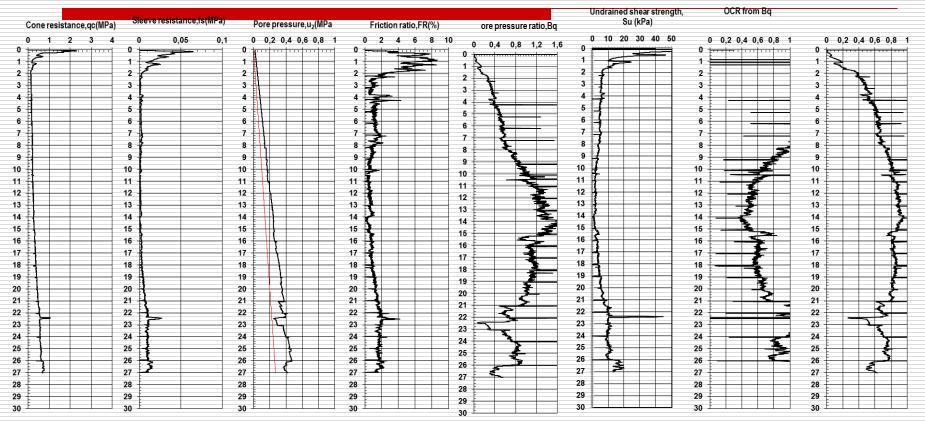
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Interpretation of CPTu-10 (center of eruption)







CASE HISTORIES

RECENT SEDIMENTS AND ORGANICS OR PEATS

SOILS THAT BECOMES SOFT UPON EXPOSURE TO WATER

SOILS THAT LIQUEFIES DURING EARTHQUAKES

1. Failures of embankment and fill placement on soft soils

 Large long term consolidation settlement
 Placement of embankment fill
 Failures of sheetpiles retaining structures



1.1. Problems of Soft Soil : Large and long term settlement

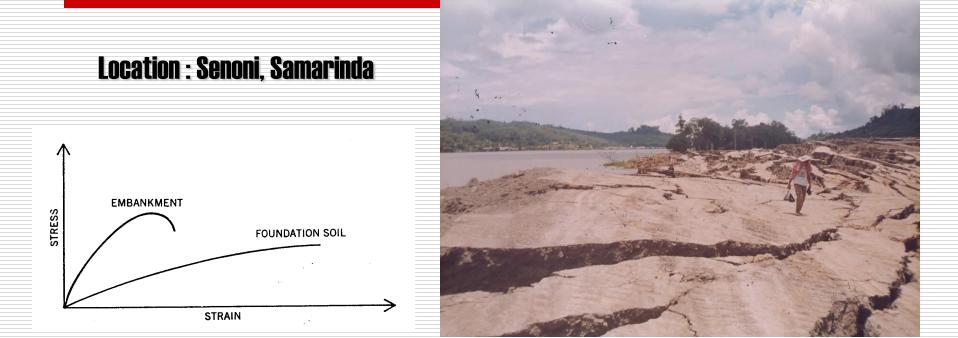


Marunda, north Jakarta

-Very common situation
fill thickness in the range of 1-5 meters
no engineering or design
mostly without soil investigation or
lack of geotechnical data

East Java

1.2.PROBLEM OF SOFT SOIL IN INDONESIA Failures of Embankment Constructed on Soft Soils



Fill Material : Well Compacted Sandy Material

Failures was deep slide type , very high pore pressures and incompatibility in stiffness of foundation soils and fill

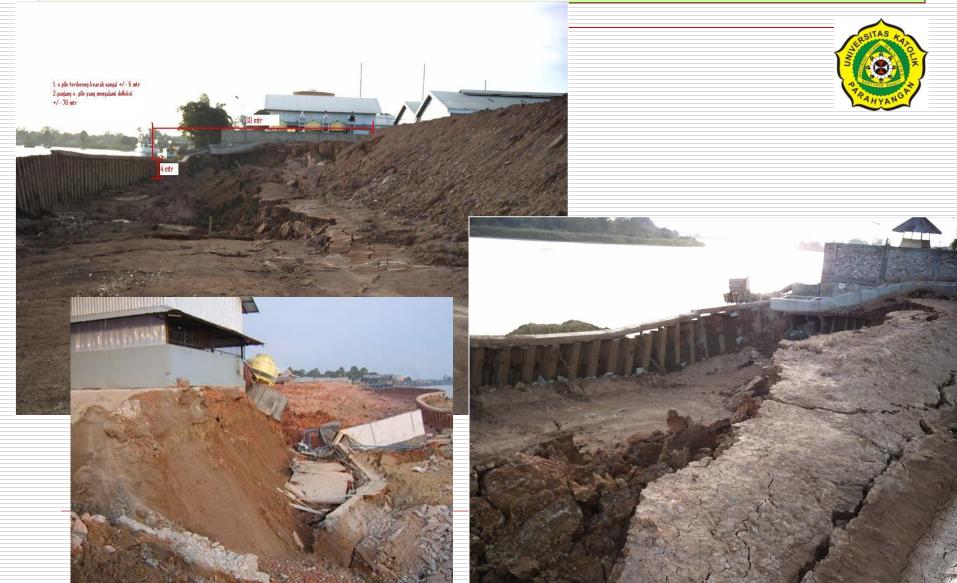
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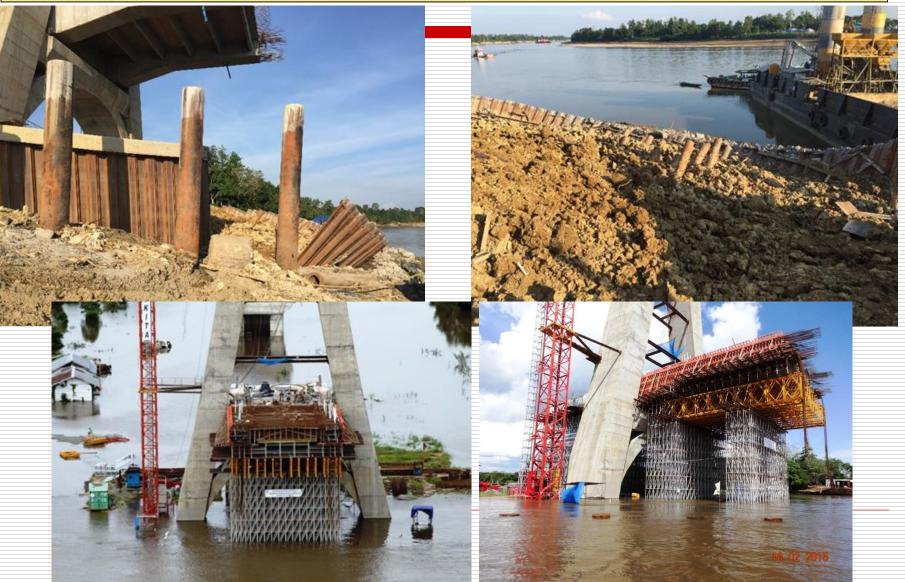
PROBLEM SOFT SOIL IN INDONESIA : 1.3. Failures of sheet piles due to excess pore water pressures resulted from fill



PROBLEM SOFT SOIL IN INDONESIA : 1.4. Failures of bridge abutment due to excess pore water pressures resulted from fill



PROBLEM SOFT SOIL IN INDONESIA : 1.5. Failures of soft soils due to sudden drawdon causing collapse of sheetpile



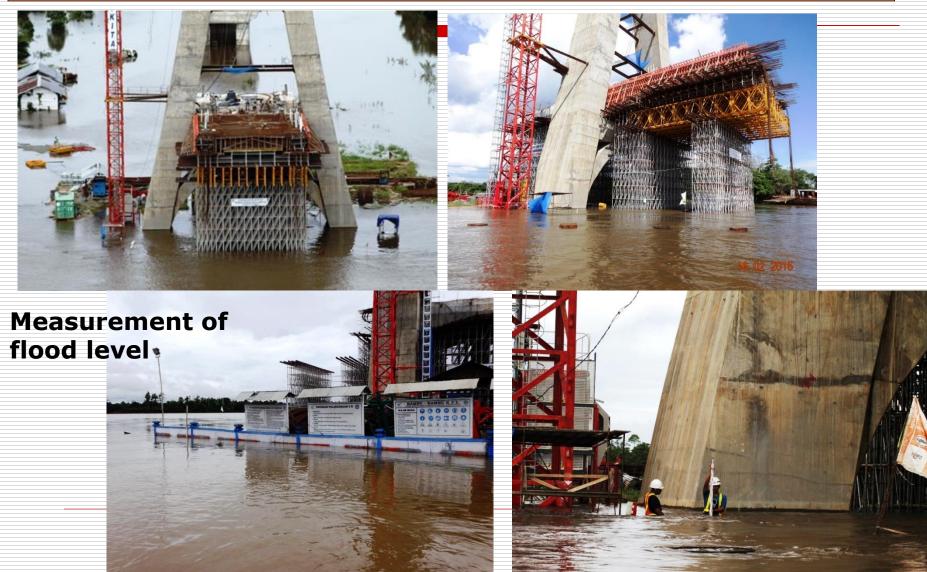
PROBLEM SOFT SOIL IN INDONESIA : 1.5. Failures of soft soils due to sudden drawdon causing collapse of sheetpile





PROBLEM SOFT SOIL IN INDONESIA : 1.5. Failures of soft soils due to sudden drawdon causing collapse of sheetpile



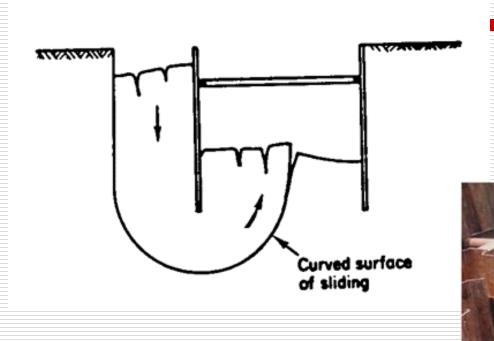


1.5.Problems of Soft Soil in Indonesia: FAILURES OF SHEET PILES WITHOUT EMBEDMENT INTO FIRM LAYER FAIL BY SUDDEN DRAWDOWN

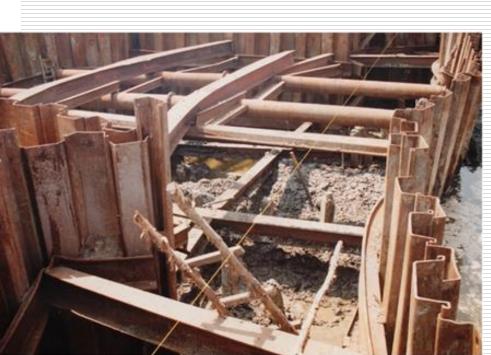
18 **Tenggarong, East Kalimantan** 20 22 24 26 28 S 18 S 16 ac (ka/cm2 120 140 22 MARTER 24 28

PROBLEM SOFT SOIL IN INDONESIA : 1.6. Failures of excavation in consolidating soils → initial pore pressure not considered





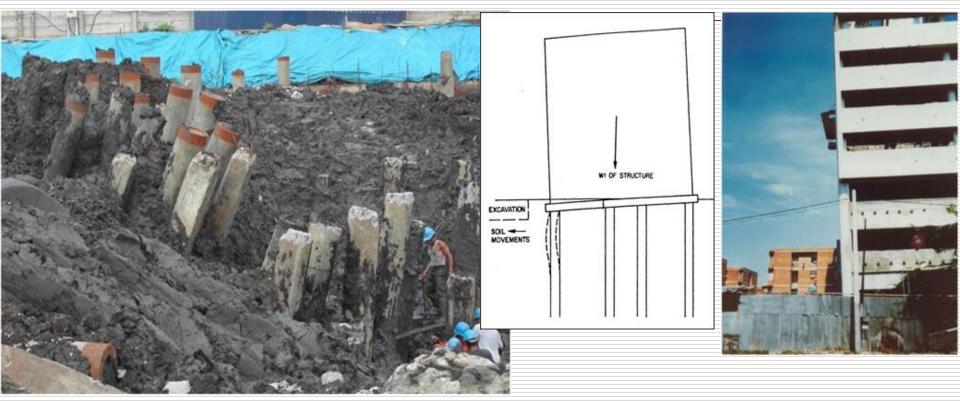
Condition During Flood



Failures of excavation in Underconsolidating soils

PROBLEM SOFT SOIL IN INDONESIA : 1.6. Failures of excavation in consolidating soils → initial pore pressure not considered





Excavation cause sliding and Tilting (and breaking) of Piles Failures of building foundation Due to excavation in Underconsolidating soils



PROBLEM SOFT SOIL IN INDONESIA : 1.6. Proper Method for Excavation in Soft Soils



Excavation in Jakarta Marine Clay

Protection Using Diaphram Wall And struts in underconsilidating soil



PROBLEM SOFT SOIL IN INDONESIA : 1.6. Proper Method for Excavation in Soft Soils



Excavation in Jakarta Soft Marine Clay

Protection Using Top Down Method



SOFT SOILS

RECENT SEDIMENTS AND ORGANICS OR PEATS

SOILS THAT BECOMES SOFT UPON EXPOSURE TO WATER

SOILS THAT LIQUEFIES DURING EARTHQUAKES

2. Soils sensitive to water or soils that become soft due to exposure to water



2.1. Soils originated from volcanic ashes2.2. Expansive soils2.3. Clayshales2.4. Uncompacted materials



2.1 ENGINEERING PROBLEMS OF CONSTRUCTION ON SOILS ORIGINATED FROM VOLCANICS ASH



 Problems with water sensitivity
 Problems with high plasticity and difficulty in compaction work



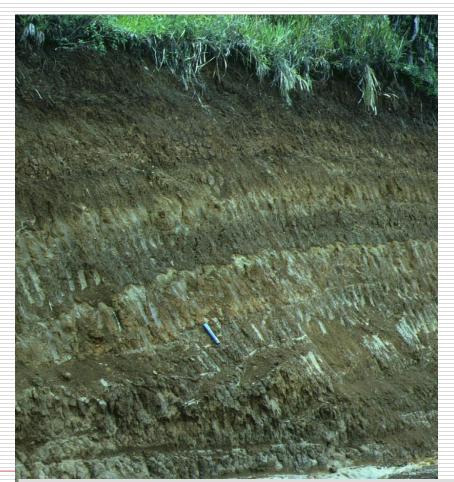
Volcanic Ashes

- strong mineralogical influence (allophane)
- some micro but no macrostructural influence

L.D. Wesley (2001)



Tuff at Sepinggan (Balikpapan)



Clay with Allophane content Geothermal Project, West Java



Problems with cut slopes



Problems of Embankment using Volcanic Ash material



Road Condition after rains

Road after construction

Timika - Papua

20/07/2012

Problems in Embankment using Volcanic Ash material





Road Condition after rains

Problems of Embankment using Volcanic Ash material







Road Condition after rains

Problems of retaining structures using tuff material for backfill



Samarinda – East Kalimantan

Road Condition after failures by rains

Problems of retaining structures using tuff material forbackfill





Samarinda – East Kalimantan

Road Condition after failures by rains



Summary on tuff and volcanic soils

- Material originated from volcanic ash are generally sensitive to the presence of water
- Problems arised in stability of cuts and fill, also on the performance of embankment
- Some methods of stabilization needed when those material should be used

Soil that becomes Soft LIQUIFACTION



- LIQUIFACTION : is phenomena where soils lost their strength and flow at constant volume, constant shear stress and constant effective stress due to increase in excess pore water pressure during earthquake.
- Liquefaction Phenomena may be sand blows, flow liquifaction/Liquifaksi or lateral spreading dan landslides) or cyclic mobility

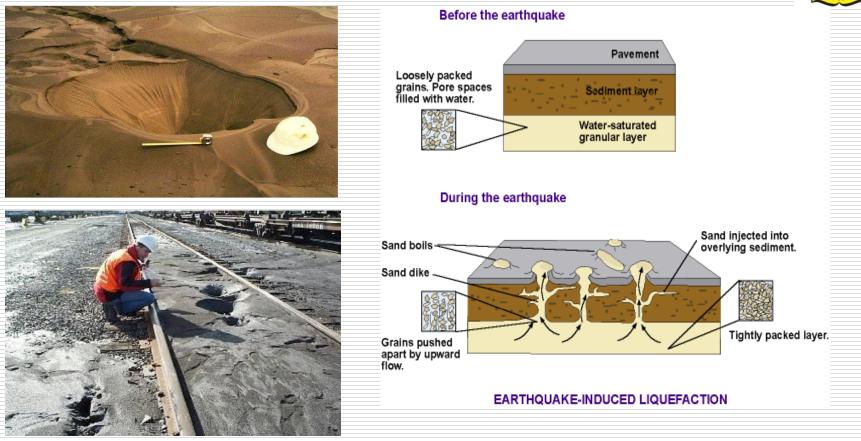
Damages



- Failures of Shallow Foundation
- Failures of deep foundation due to degradation of strength and lateral spreading
- Foundation displacement
- Failures of retaining structures
- Large scale Settlement and heave
- Lateral spreading
- Landslides and flow liquifaction

Phenomena of sand blows





Phenomena of Flow Liquefaction (Satelite image of Petobo – Palu Donggala Eartquake)







neighbourhood

Palu Bav

Petobo ub-district

Jono Oge village

Damage due to Liquifaction - Petobo : 744 houses

- Balaroa : 1700 houses
- Jono Oge :



Massive Liquefaction Petobo village and Perumnas Balaroa

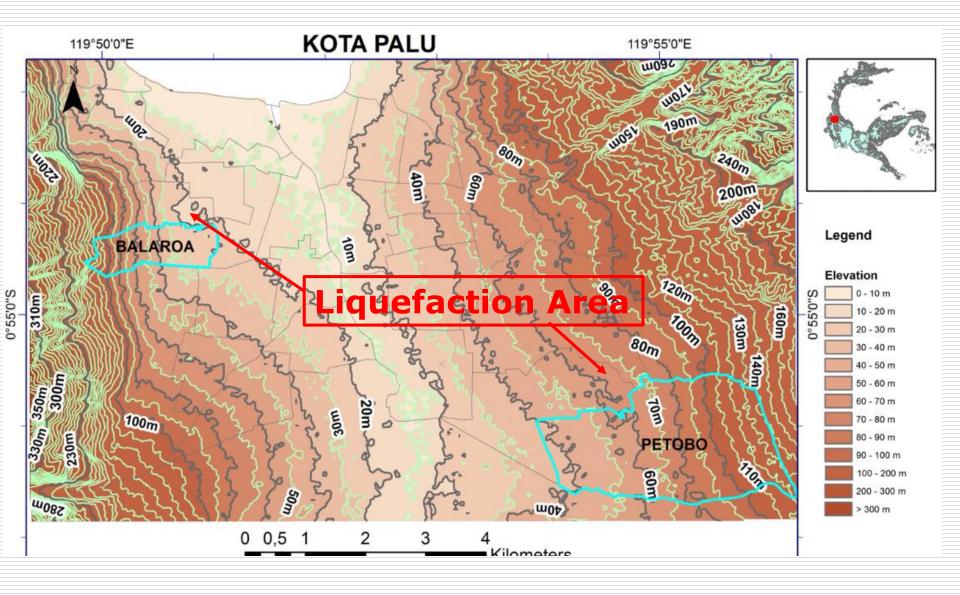


Balaroa located at Palu-Koro Fault. After liquefaction, some area settled 5 m, and other area heave 2m 1.747 units of houses damage or gone

Petobo, hundreds of houses sink into mud 3-5 m deep 744 units of houses damage or gone



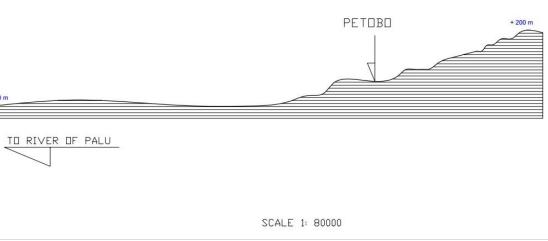






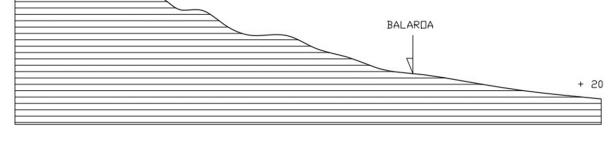








TO RIVER OF PALU





SCALE 1: 50000



Scenes at Petobo









Scenes in Petobo (photo on Oct 1, 2018)



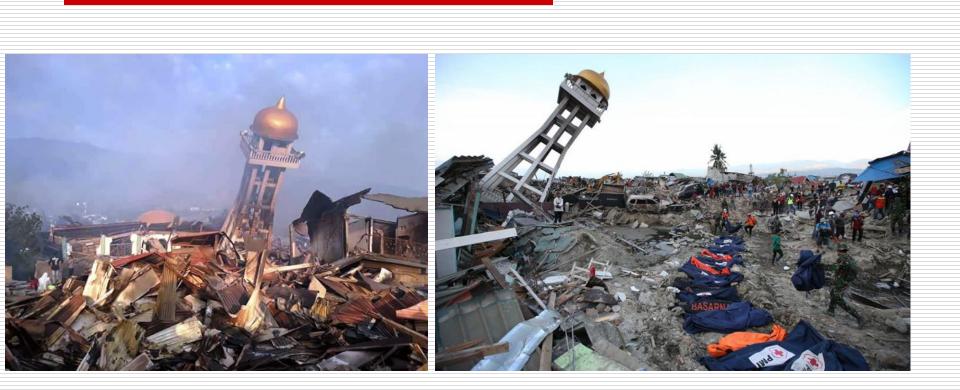
Scenes at Petobo



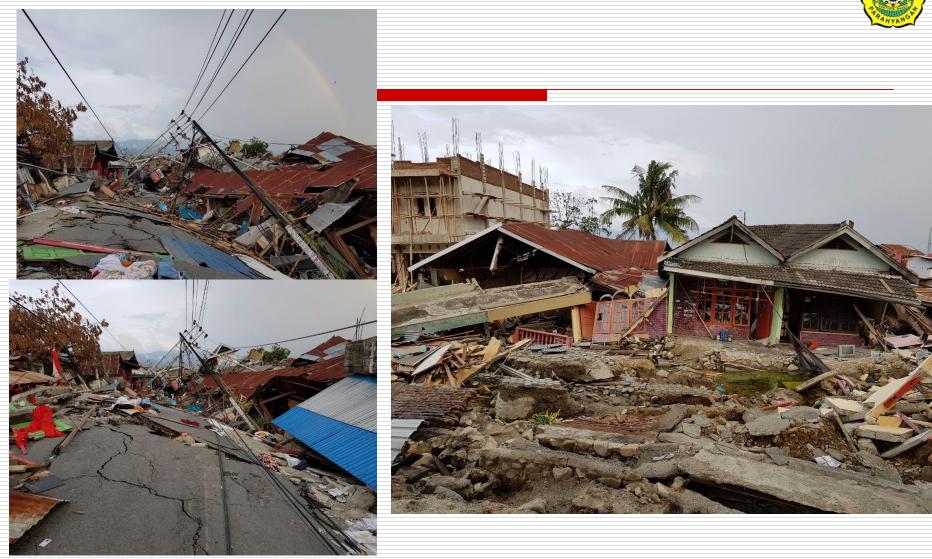


Liquifaction at Balaroa - Palu, Central Sulawesi Thirsday, (foto on 11 Oktober 2018)









Liquefaction at Desa Lolu Kecamatan Sigi (foto on oct 5, 2018)





Evacuation of the liquefaction area





Evacuation of Liquefaction debris



Conclusions

- Soft soils origin and phenomena have been presented, there are originally deposited soft soils and soil that becomes soft due to water or excess pore water pressures
- There have been many cases which are very valuable as lessons learned and should be very good reference