## Mega Infrastructure Soil Improvement Technology and Case Study

Liu Yu Geoharbour Group



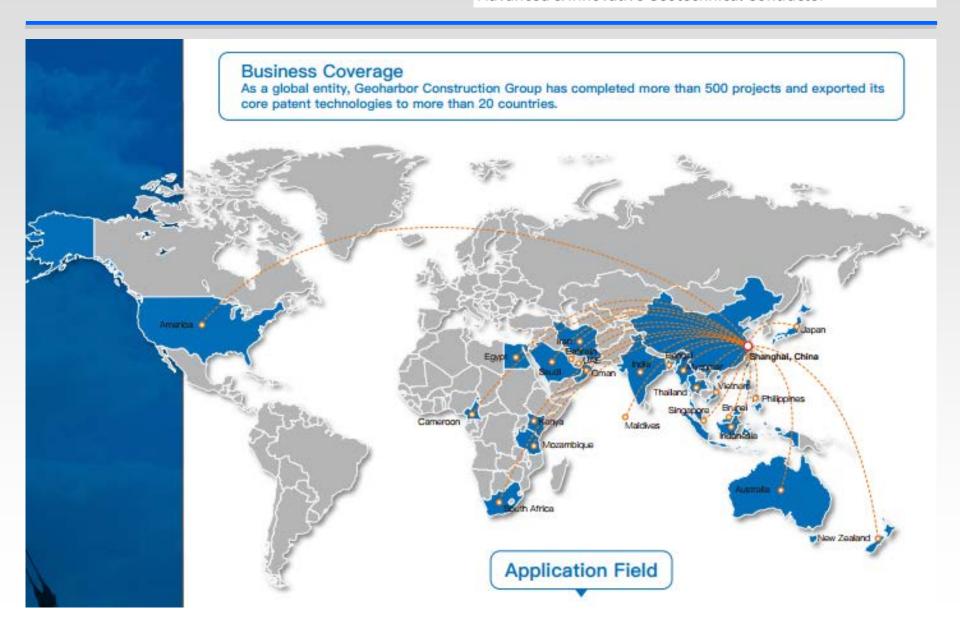
#### **GEOHARBOUR CONSTRUCTION GROUP**

Advanced & Innovative Geotechnical Contractor

- Founded Date: Jan. 28th, 2000.
- Professional entity specializing in Geotechnical Research, Design,
   Construction and Consultancy.
- Chief Editor of Chinese National Code for Reclamation Soil Treatment.
- Director member of Technical Committee of China Soft Soil Treatment.
- Vice President of China Association of Inventions.
- More than 30 patents for ground improvement method.
- Over 10 subsidiary companies: Singapore, Indonesia, Malaysia, Vietnam, Myanmar, Thailand, Middle-East Area, Australia, Panama, India, Iran...
- Over 500 large-scale projects (including China).
- Soft Soil Treatment more than 15,000 Ha (Include China).
- 50+ ongoing projects.

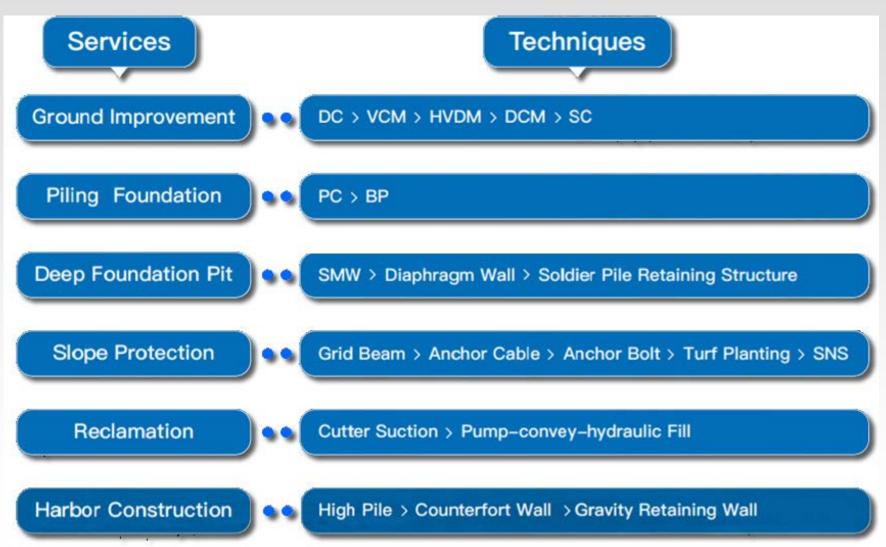
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## PEMBANGUNAN INFRASTRUKTUR





- Jalan baru 2,650 Km
- Jalan tol 1.000 Km
- Pemeliharaan jalan 46.770 Km



 Pembangunan Jalur KA 3,258 Km di Jawa, Sumatera, Sulawesi dan Kalimantan terdiri dari: KA Antar kota 2,159 Km, KA Perkotaan 1,099 Km



- Pembangunan 15 Bandara baru
- Pengadaan 20 Pesawat Perintis
- Pengembangan Bandara untuk
- pelayanan Cargo Udara di 6 Lokasi



- Pembangunan Pelabuhan
   Penyeberangan di 60 Lokasi
- Pengadaan Kapal penyeberangan perintis sebanyak 50 unit



- Pembangunan 24 Pelabuhan baru
- Pengadaan 26 Kapal Barang Perintis
- Pengadaan 2 Kapal Ternak
- Pengadaan 500 unit Kapal rakyat



- Pembangunan BRT di 29 kota
- Pembangunan angkutan massal cepat di kawasan perkotaan (6 Kota metropolitan, 17 Kota besar)



- Soft clay area: west part of Sumatera, north part of Java, most of Kalimantan, south part of Papua
- Loose sand area: south and north part of Sumatera, part of Sulawesi, NTT, NTB,
   Maluku







Palembang – Indralaya Toll Road, South Sumatera

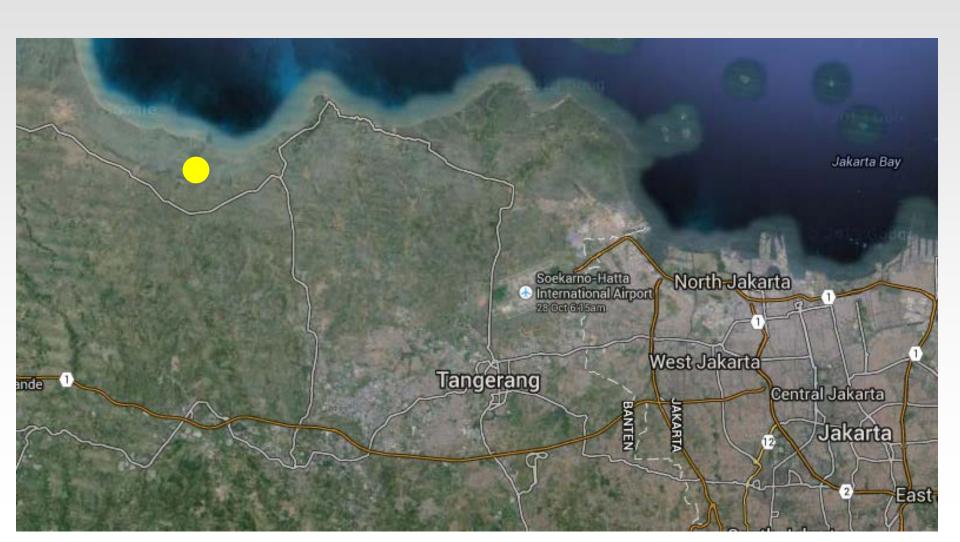








## PLTU III Project, Teluk Naga, Tangerang, Banten (2008)





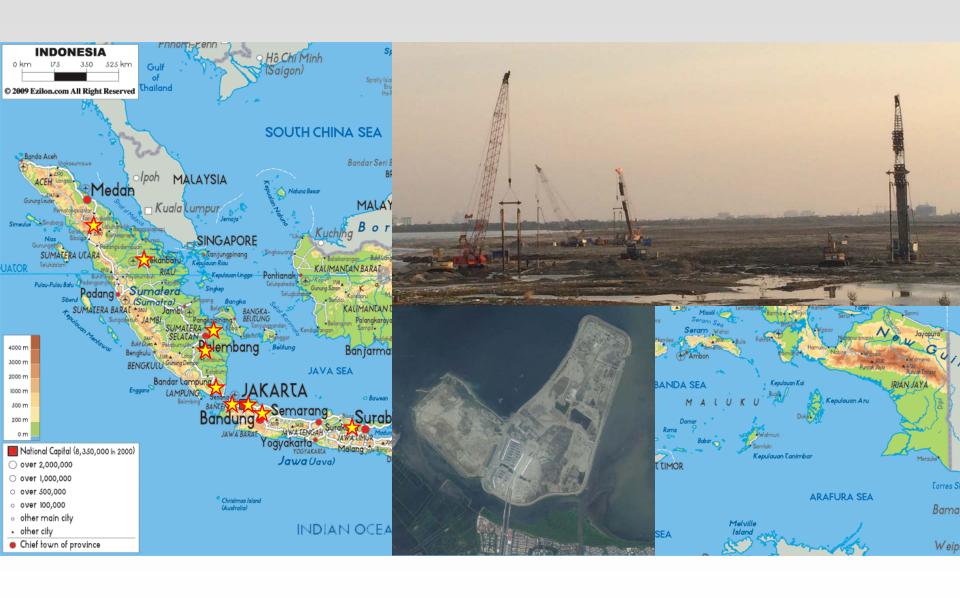
## PLTU III Project, Teluk Naga, Tangerang, Banten (2008)



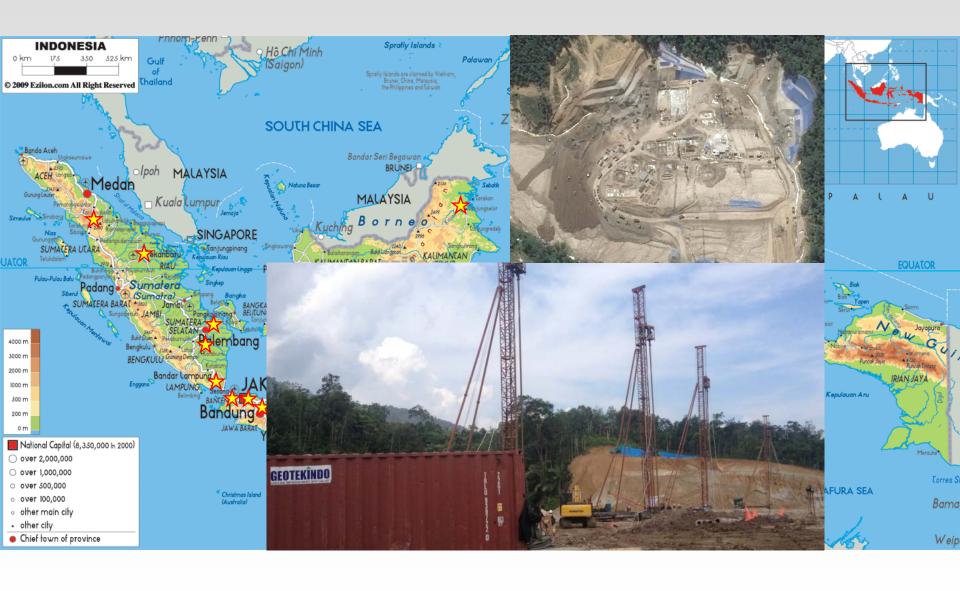


























# Case 1 Jawa 7 Power Plant Project



#### Jawa 7 Power Plant Project





#### Jawa 7 Power Plant Project



#### Challenge:

- Residual settlement shall less than 10 cm.
- > The power plant shall be built in 3 years.
- Ground surface is very soft or extremely soft.
- Traffic limitation for soil/sand transportation.

Soft Clay Parameter:

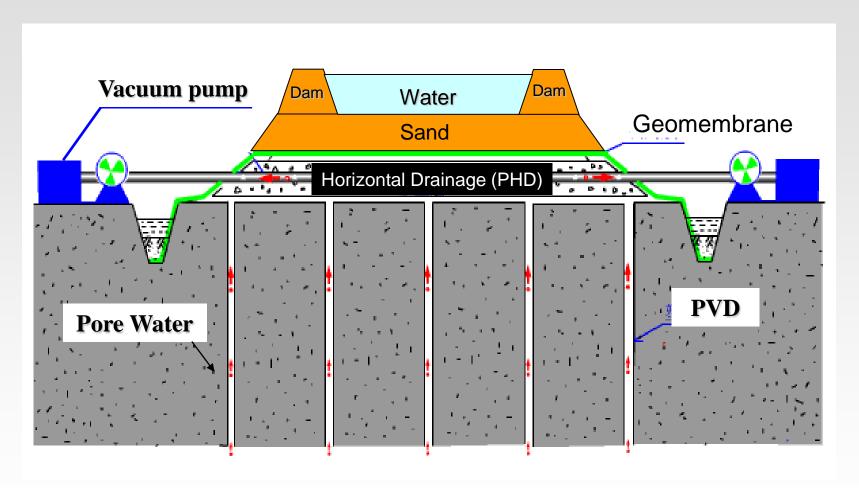
Initial Water Content 100%~140%

Undrain Shear Strength: 5.3 kPa

Thickness: 6~18 meter



Jawa 7 Power Plant Project – VCM + Water Surcharge



Vacuum System provided 80 kPa preload Water surcharge provide additional 20 kPa preload Sand backfilling is for leveling purpose

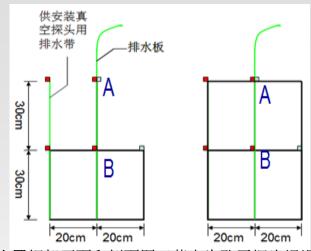


## **Indoor Trial Test**



## **Indoor Trial Test**





内置钢架正面和侧面图 (蓝点为孔压探头埋设处)

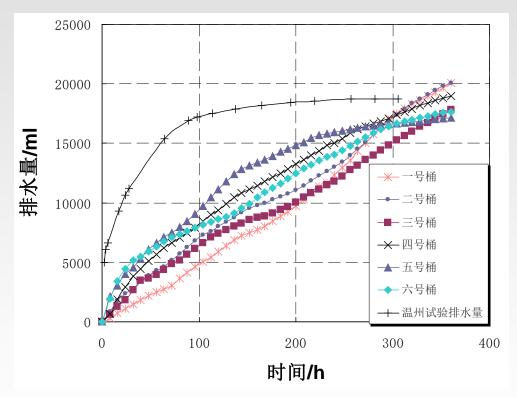






#### **Indoor Trial Test**

试验桶编号 NO.	1	2	3	4	5	6
直径Diameter(m)	1.2	1.2	1.0	1.0	0.8	0.8



#### Main Conclusions of Indoor Trial Test:

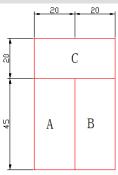
- > No clogging was found
- Proper PVD spacing is 1.0 m



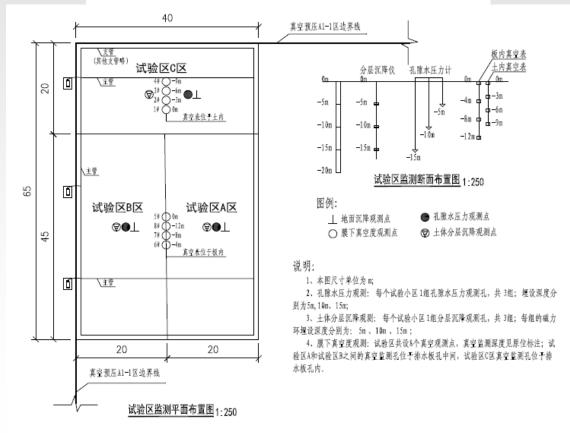




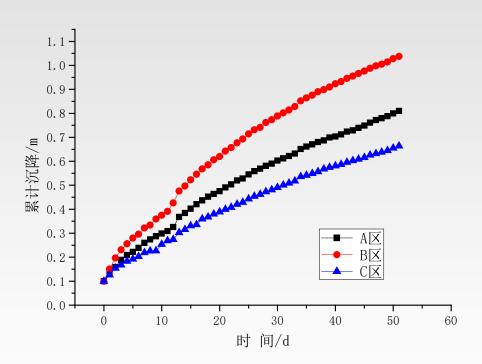




区域名称	排水板滤膜尺寸(um)	排水板间距(m)
A	120	1.0
В	120	0.8
С	120	1.2







Observed Settlement vs Time During In-situ Trial Test

Due to time limitation, the in-situ trial test area keep running for 51 days. The main conclusions are:

- 1, Settlement in Test Zone-A is 0.810 m, average consolidation degree is 53.3%; Settlement in Test Zone-B is 1.037 m, average consolidation degree is 67.1%; Settlement in Test Zone-C is 0.664 m, average consolidation degree is 37.1%; Vacuum running well. Settlement keep growing and none clogging was found.
- 2. Spacing 1.2 m result obviously longer consolidation time which was not suitable for this case. Spacing 1.0 m is more proper.



## **Construction Steps**



Jawa 7 Power Plant Project – VCM + Water Surcharge





Jawa 7 Power Plant Project – VCM + Water Surcharge





Jawa 7 Power Plant Project – VCM + Water Surcharge





### Jawa 7 Power Plant Project – VCM + Water Surcharge





## Jawa 7 Power Plant Project – VCM + Water Surcharge











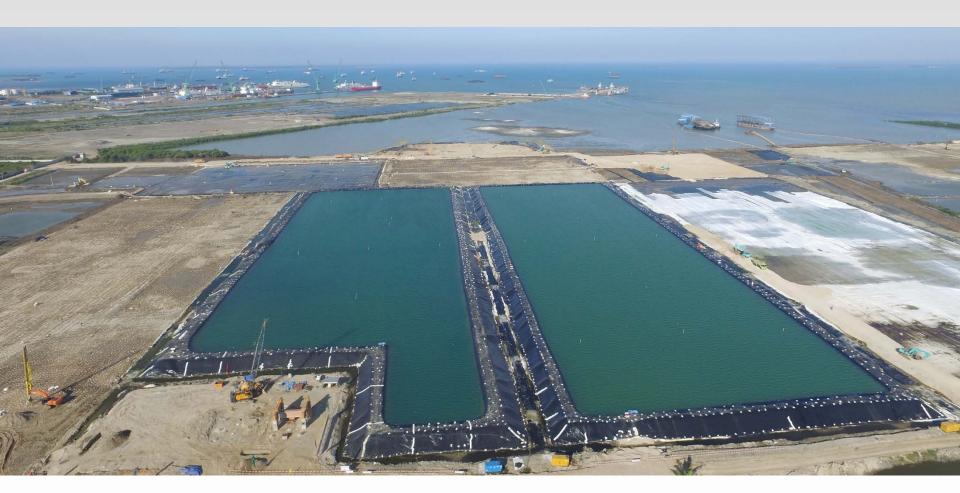












After VCM + Water Surcharge improvement, the ground settle 1.4~1.5 m and the unconfined shear strength Su increase 18~25 kpa





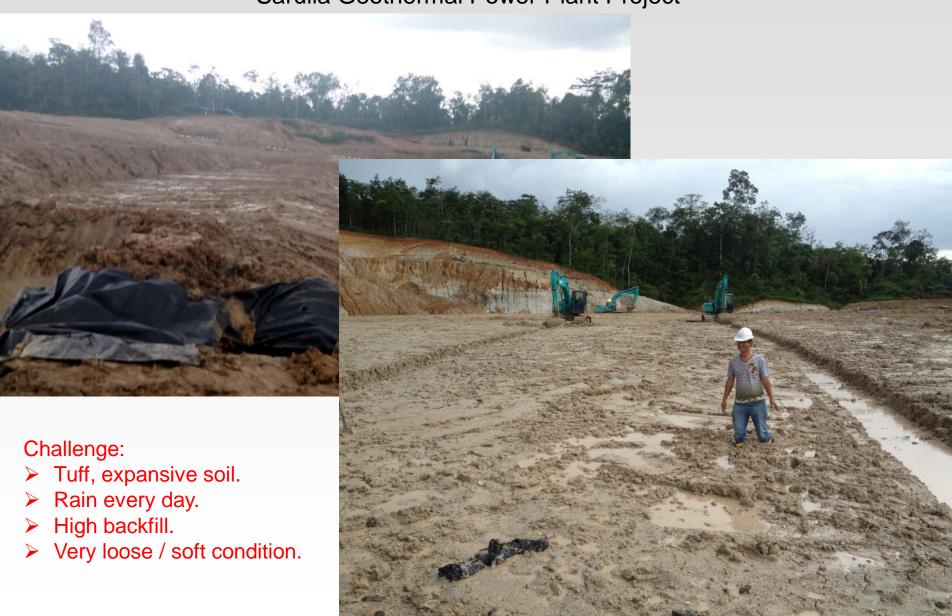
Excavation up to 6.7 m without retaining wall













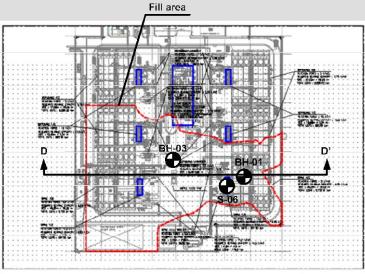


Figure 2. Layout of the Sil Area at Sarulla Power Plant

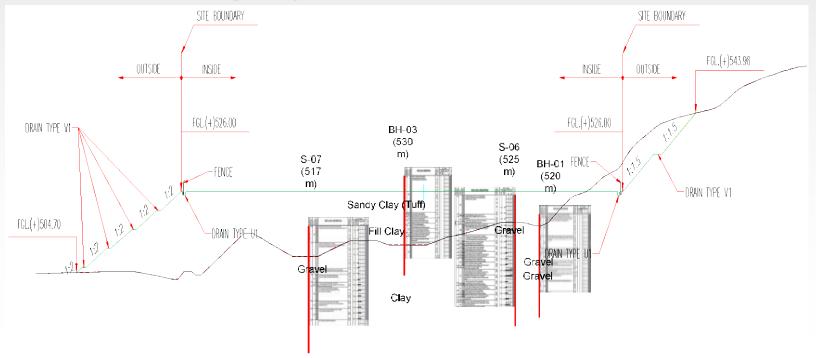




Table 4. DCM trial mixing laboratory result (Data from PT. Soilens)

Borhole	Depth of sample (m)	Cement Ratio (%)	Curing Time (days)	UCS, qu (kg/cm²)	Modulus, E (kg/cm²)	E/qu	Borhole	Depth of sample (m)	Cement Ratio (%)	Curing Time (days)	UCS, qu (kg/am²)	Modulus, E (kg/cm²)	E/qu
		9	7	18.50	2000	108		0.00 - 3.00	9	7	24.33	2200	90
			14	19.93	1875	94				14	29.68	2000	67
			28	21.25	1875	88				28	30.32	1500	49
		11	7	26.35	3600	137			11	7	33.94	2500	74
			14	32.40	2000	62				14	34.15	1875	55
	0.00 - 2.50		28	36.26	1875	52				28	38.99	2500	64
		13	7	29.00	5000	172			13	7	40.67	3125	77
			14	32.28	2750	85	1			14	42.18	2500	59
			28	41.13	1800	44				28	43.15	3125	72
		15	7	32.25	2500	78	1			7	44.19	2500	57
			14	35.33	2000	57	1			14	47.29	2750	58
			28	43.64	2400	55	1			28	52.33	3000	57
			7	40.39	3000	74	1		17	7	45.08	3000	67
		17	14	44.15	3000	68	1			14	48.78	2500	51
		''	28	45.97	2000	44	1			28	53.99	3250	60
			7	15.37	2000	130	1			7	20.83	3000	144
		9	14	17.38	1375	79	1		9	14	22.47	2500	111
			28	21.76	1500	69	1			28	26.51	2000	75
	2.50 - 5.00	11	7	28.38	4250	150	-			7	28.70	2250	78
			14	32.40	2500	77	1			14	32.97	2250	68
			28	38.03	3000	79				28	35.00	3000	86
		13	<u> </u>	33.49	4000	119	1	3.00 - 6.00	13	<u> </u>	31.29	1400	45
BH-1			14	39.64	3000	76	ВН-3			14	35.73	1500	42
			28	47.43	3000	63				28	46.25	2250	49
			7	37.57	3500	93			15	7	33.07	4250	129
		15	14	45.32	2750	61				14	40.98	2200	54
			28	52.30	3000	57				28	46.58	3250	70
		17	7	40.17	5500	137				7	39.02	2750	70
			14	47.94	3250	68	1			14	46.57	2000	43
			28	54.68	3500	64				28	47.38	2200	46
			7	22.13	2000	90				7	20.69	2250	109
		9	14	28.52	2250	79		6.00 - 9.00	9	14	27.60	2250	82
			28	30.25	2750	91				28	31.63	3000	95
		11	<u> </u>	28.98	3000	104			11 13	<u> 26</u> 7	24.69	3000	122
	5.00 - 7.50		14	32.28	2500	77				14	30.63	2000	65
			28	36.25	2500	69				28	33.09	2000	60
		13	<u> </u>	34.51	3500	101					33.00	2375	72
			14	36.75	2250	61				14	36.68	2750	75
			28	38.37	3000	78				28	40.91	2500	61
		15	<u> </u>	35.44	3000	85				<u> </u>	35.07	2500	71
			14	39.00	1875	48				14	38.97	2500	58
			28	47.38	2500	53				28	38.97 44.29	2750	62
			<u> 28</u> 7	47.38 37.64	6000	159				<u> 28</u> 7	37.74	2500	66
		17		42.05	2250				17	14			
			14 28		2500	<u>54</u> 51				14 28	42.52	2000	47 45
			<b>Z</b> 8	48.78	∠500	31				25	53.15	2400	45

Cement Ratio 9% is acceptable.



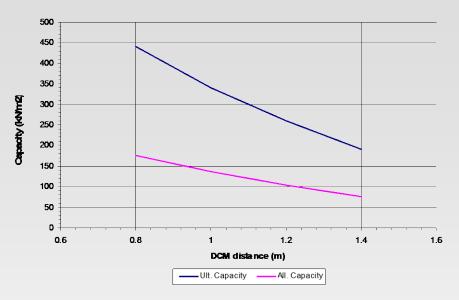


Figure 14. DCM distance vs Capacity

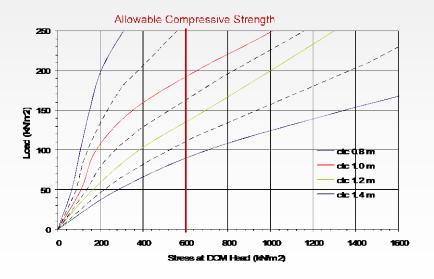


Figure 15. Stress at DCM head vs Load

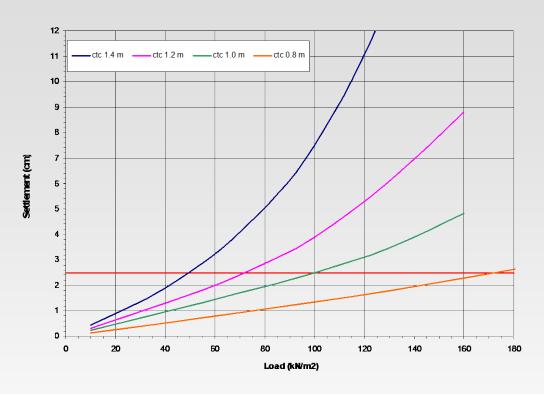


Figure 16. Load vs Settlement

Table 7. Allowable load for 25 mm settlement

DCM Spacing (m)	Allowable Load For 25 mm Settlement (kN/m²)
0.8	175
1.0	100
1.2	74
1.4	48



#### Table 8. DCM Spacing Sarulla Power Plant - SIL area



Figure 20. Load Zone for SIL area

Table 6. De M. Spacing Saruna rower Flant - SIL area										
Name			Foundation	Max. Reaction	Quantity	DCM Spacing	Est. Stress at DCM	Load for each DCM (kN)		
	ID	Name	Size (m)	(kPa)	(EA)	With Raft (m)	Head (kN/m²)			
	6	Brine Acc	49.02 x 39.00 x 0.40	28.3	1	1.3*	120	46		
Zone 1	7	Brine Elec. Shelter	3.30 x 2.60 x 0.40	34.7	1	1.3*	145	56		
	,	BIANG 1200. BAGICAL	4.60 x 2.30 x 0.40	34.7	1	1.3*	145	56		
	8		4.00 x 4.00 x 0.70	87.8	2	1.1	250	96		
		Brine Vaporizer	4.30 x 4.00 x 0.70	72.1	1	1.2	260	100		
			4.70 x 3.20 x 0.70	78.8	1	1.1	200	77		
	9	Brine Feed Pump	5.16 x 2.60 x 0.40	36.2	4	1.3*	116	45		
	6	Brine Acc	49.02 x 39.00 x 0.40	28.3	1	1.3*	120	46		
Zone 2	7	Brine Elec. Shelter	3.30 x 2.60 x 0.40	34.7	1	1.3*	145	56		
		Brine Elec. Sherier	4.60 x 2.30 x 0.40	34.7	1	1.3*	145	56		
		Brine Vaporizer	4.00 x 4.00 x 0.70	87.8	2	1.1	250	96		
	8		4.30 x 4.00 x 0.70	72.1	1	1.2	260	100		
			4.70 x 3.20 x 0.70	78.8	1	1.1	200	77		
	9	Brine Feed Pump	5.16 x 2.60 x 0.40	36.2	4	1.3*	116	45		
	1	Bottoming ACC	57.00 x 39.00 x 0.40	22	1	1.3*	85	33		
	2	Bottom. Cond.	4.40 x 4.40 x 1.20	76.1	2	1.1	194	75		
		Bottom. Elec.	3.30 x 2.60 x 0.40	33.4	1	1.3*	92	35		
	3	Shelter	4.30 x 2.20 x 0.40	33.9	1	1.3*	92	35		
	4	Bottoming	6.20 x 3.80 x 0.70	73	2	1.2	255	98		
Zone 3		Vaporizer	6.40 x 4.30 x 0.70	79.1	2	1.1	198	76		
	5	Bottom. Feed	2.80 x 2.60 x 0.60	97	2	1.0	178	69		
			3.70 x 3.70 x 0.40	63	2	1.3	270	104		
	10	Cooling Water Air Cooler	3.70 x 2.40 x 0.40	63	6	1.3	270	104		
		Coole	2.40 x 2.40 x 0.40	63	4	1.3	270	104		
Zone 4	1	Bottoming ACC	57.00 x 39.00 x 0.40	22	1	1.3*	85	33		
	2	Bottom. Cond.	4.40 x 4.40 x 1.20	76.1	2	1.1	194	75		
	3	Bottom. Elec.	3.30 x 2.60 x 0.40	33.4	1	1.3*	92	35		
		Shelter	4.30 x 2.20 x 0.40	33.9	1	1.3*	92	35		
		Bottoming	6.20 x 3.80 x 0.70	73	2	1.2	255	98		
	4	Vaporizer	6.40 x 4.30 x 0.70	79.1	2	1.1	198	76		
	5	Bottom. Feed	2.80 x 2.60 x 0.60	97	2	1.0	178	69		
Zone 5	1	Bottoming ACC	57.00 x 39.00 x 0.40	22	1	1.3*	85	33		
	5	Bottom. Feed	2.80 x 2.60 x 0.60	97	2	1.0	178	69		



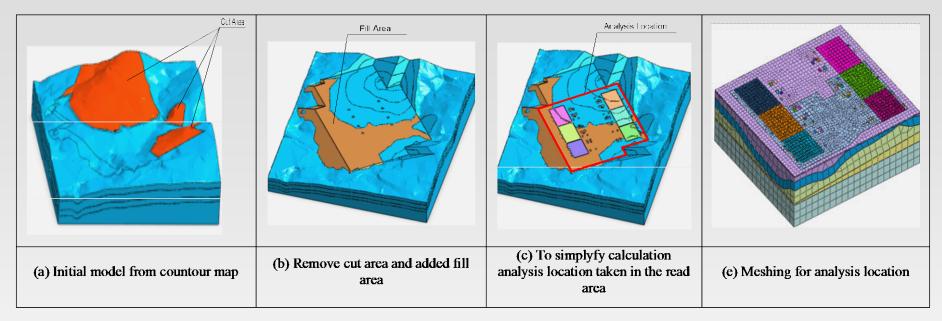


Figure 21. 3D modeling step for full scale model

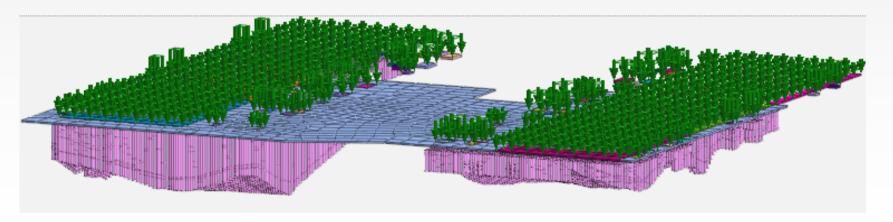


Figure 22. Raft, Cushion, DCM, & Load model



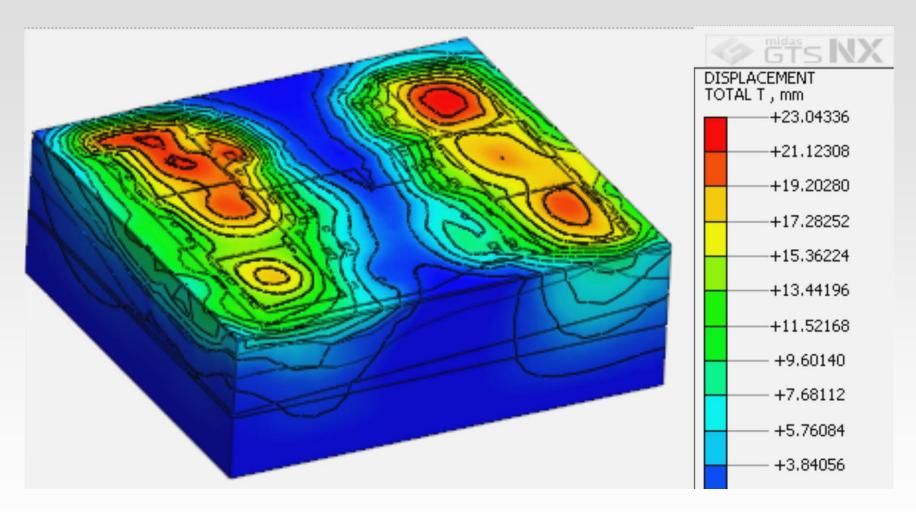
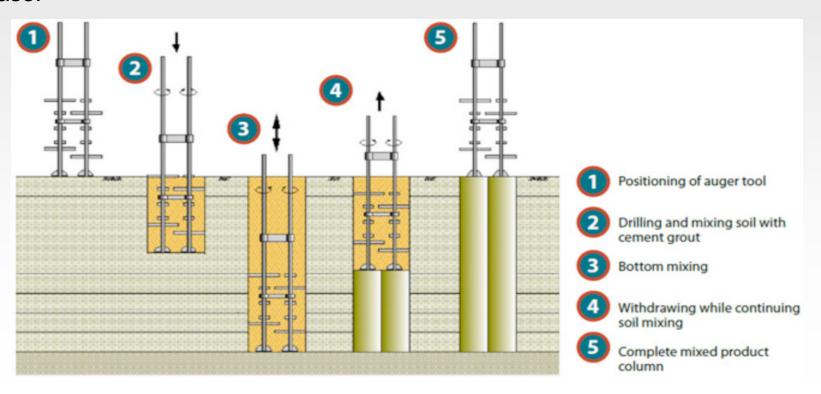


Figure 24. Analysis result for full scale model



Deep cement mixing (DCM) is a geotechnical technique where a binder material, typically cement, is injected into the ground for ground stabilization and land reclamation. In ground stabilization applications it is typically used to obtain a better load bearing capability of the existing soil, e.g. in order to bear buildings and other structures. In land reclamation applications it is typically used when cheaper techniques such as dredging or draining cannot be applied because of environmental concerns due to contaminated soil that these two techniques would release.

















Site Condition after DCM Soil Improvement



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# Thank you for your attention.

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